

19,338.39 M

128,164.71 M

DRILLED BY Metropolitan SMT BORING NUMBER C-90

CHECKED BY LDW Drilled with Air Between Depths of 0 to 60' Drilled with Water Between Depths of 60' to 127.5' Boring grouted to Surface upon completion.

DATE STARTED

3/13/84 DATE COMPLETED 3/19/84

JOB NUMBER

GS-3223.55

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	ELEVFT	DEPTH FEET		ORM	TIO	N	0	PENE 5		10N- 15 2	BLOW:	5 PER		80	100
	339.0	40	WHITE TO GRAY FINE SANDY SILTY LIMESTONE, MODERATELY WELL JOINT- ED, IRON STAINING, SOME CLAY LIN- ING ON JOINT SURFACES	-	NA GOOD	1 R	5' C 9' C								
:	329.0	50				1.	5 '								75/6
			VERY HARD TAN TO WHITE FINE SANDY SILTY CLAY, POORLY CEMENTED												
	324.0						-		_						75/6
1															
	319.0	60	GRAY TO TAN FINE SANDY SILTY CLAY-	IA DIAZ		REC	-		+		-		$\frac{1}{1}$		
	314.0		EY LIMESTONE, MODERATELY WELL- JOINTED WITH OCCASIONAL CLAY SEAMS AND IRON STAINING	1 7	CORE	2.3									
	314.0					REC 2.0			-			$\frac{ \cdot }{ \cdot }$	-		
3	09.0					REC 1.8									
					RE NX	REC	-			+					
30	04.0			•	8	REC 2.3'									
								-		+-			+		
	9.0 MARKS:	80													

DRILLED BY Metropolitan SMT BORING NUMBER C-90
LOGGED BY MJC DATE STARTED 3/13/
CHECKED BY LDW DATE COM-

BORING NUMBER C-90
DATE STARTED 3/13/84
DATE COMPLETED 3/19/84
JOB NUMBER GS-3223.55

المناوات المناع	DEPT		FORMA	O NOITA	PENETRATE 5 10	ON-BLOWS 15 20 30	10 10
299.0		GRAY TO TAN FINE SANDY SILTY CLAYEY LIMESTONE, MODERATELY WELL JOINTED, WITH OCCASIONALLY CLAY SEAMS AND IRON STAINING		REC 1.5'			
289.0							
279.0	99	TAN FINE SANDY SILTY LIMESTONE, MODERATELY WELL JOINTED HORIZON- TALLY @ 45°, MODERATELY WELL CEMENTED, SOME CALCITE LINED JOINT SURFACES	JUANA DIAZ	REC 2.2'			
269.0				REC			
264.9				1.4			
259.0	120 5 :						

DRILLED BY Metropolitan SMT LOGGED BY MJW CHECKED BY LDW

BORING NUMBER C-90 DATE STARTED 3/13/84 DATE COMPLETED 3/19/84 JOB NUMBER

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	DEPTH	DESCRIPTION	FORM	ATI	NC	PENETS	RATION-BL 10 15 20	OWS PER	FOOT 60 80 1
259.0	120	TAN FINE SANDY SILTY LIMESTONE MODERATELY WELL JOINTED HORIZON- TALLY @ 45°, MODERATELY WELL CEMENTED, SOME CALCITE LINED JOINT SURFACES	NA DIAZ						
	127.5	GRAY CLAYEY LIMESTONE WITH VERTICAL JOINTS	JUANA		EC9'				
249.0		BORING TERMINATED @ 127.5 feet							
æ									

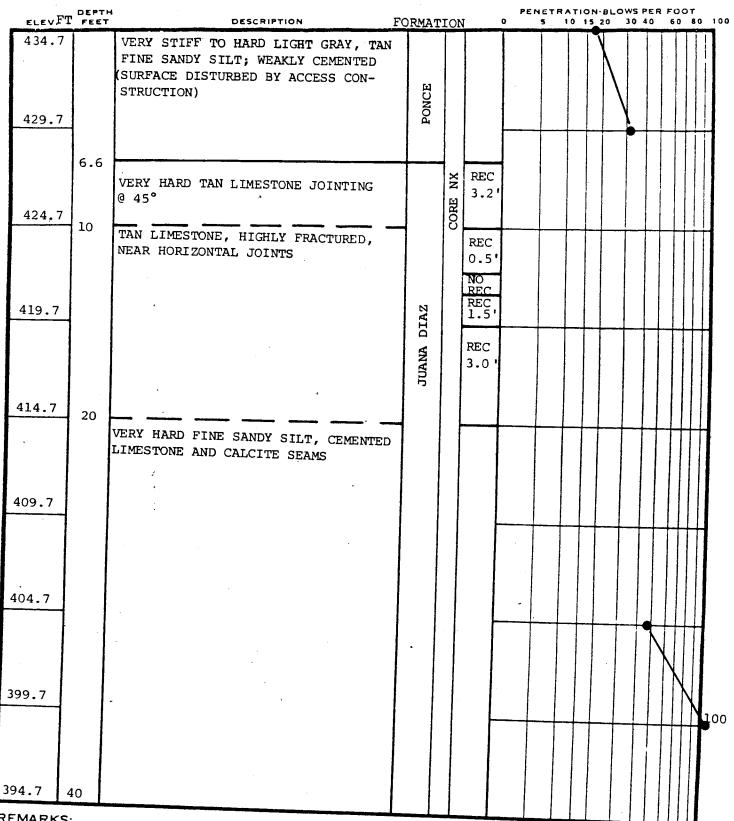
DRILLED BY Metropolitan SMT BORING NUMBER C-90
LOGGED BY MJC DATE STARTED 3/13/

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DATE COMPLETED 3/19/84

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REMARKS:

Coordinates:

N: 19,432.02m

E: 128,111.98m

Drilled with air between depths 0 to 115' Drilled with water between depths 115 to 158' Boring grouted to surface upon completion

DRILLED BY Caribbean ST LOGGED BY MJC

CHECKED BY LDW

BORING NUMBER C-91 DATE STARTED 3/13/84 DATE COMPLETED 3/16/84 JOB NUMBER GS-3223.55

ELEVF	DEPT	DESCRIPTION	FORM	ATIC	N		ATION-BI	OWS PER	FOOT 100
394.7	40	WHITE FINE SANDY SILTY LIMESTONE MODERATELY WELL JOINTED, CLAY CO ING ON SOME JOINT SURFACES, SOME IRON STAINING	VER-			REC 2.0' REC 1.9'			
384.7	50	VERY HARD TAN, GRAY FINE SANDY S		A DIAZ	CORE NX	REC 4.3'			
379.7		CLAY, POORLY CEMENTED, LIMESTONE SEAMS & FRAGMENTS	TLTT	JUANA					00
374.7									100
369.7	·	, , , , , , , , , , , , , , , , , , ,							100/
364.7									100/
359.7									100,
354.7 REMARKS	80								

REMARKS:

DRILLED BY Caribbean ST MJC CHECKED BY LDW

BORING NUMBER C-91

DATE STARTED 3/13/84

DATE COMPLETED 3/16/84

JOB NUMBER GS-3223_55

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	FTFEE	H DESCRIPTION	FORM	IATI	ON		0	ENETF 5		30 40		100
354.	7 80	GRAY, TAN FINE SANDY SILTY LIMEST WELL CEMENTED MODERATELY WELL JOI ED HORIZONTALLY				REC 5.0'						
349.7	,				XX							
					CORE	REC 5.0'						
344.7	90											75
		VERY HARD GRAY TO GREEN FINE SAND SILTY CLAY, WITH LIMESTONE FRAG- MENTS	Y									
339.7		,		DIAZ								
				JUANA D		٠						10c
334.7]			.,								
	100	TAN LIMESTONE POSSIBLY HIGHLY FRACTURED				NO REC						
329.7	1											
324.7	110											
		VERY HARD GRAY TO GREEN FINE SANDY SILTY CLAY				EC .2'						
319.7	114.2				ž							
		GRAY TO TAN FINE SANDY SILTY LIME- STONE WELL CEMENTED WITH SOME IRON STAINING		adop		EC .2'						
314.7	120				一	-						
REMARK	S:			\perp								

REMARKS:

DRILLED BY Caribbean ST
LOGGED BY MJC
CHECKED BY LDW

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APPENDIX 6

Report of Geotechnical Data From Borings C-88, C-89, C-90, C-91 Ponce Waste Facility Ponce, Puerto Rico F - SUMMARY DESCRIPTION OF CONTAMINANT TRANSPORT MODEL

SUMMARY DESCRIPTION OF CONTAMINANT TRANSPORT MODEL (Abstracted From Konikow and Bredehoeft)

The model calculates the transient changes in the concentration of a nonreactive solute in flowing ground water. The computer program solves two simultaneous partial differential equations. One equation is the ground-water flow equation, which describes the head distribution in the aquifer. The second is the solute-transport equation, which describes the chemical concentration in the system. By coupling the flow equation with the solute-transport equation, the model can be applied to both steady-state and transient flow problems.

The purpose of the simulation model is to compute the concentration of a dissolved chemical species in an aquifer at any specified place and time.

The equation describing the transient two-dimential areal flow of a homogeneous compressible fluid through a nonhomogeneous anisotropic aquifer can be written in Cartesian tensor notation as

$$\frac{\partial}{\partial x_{i}} \left(T_{ij} \frac{\partial h}{\partial x_{j}} \right) = S \frac{\partial h}{\partial t} + W \qquad i, j = 1, 2$$
 (1)

Where

is the transmissivity tensor, L²/T;

h is the hydraulic head, L;

s is the storage coefficient, (dimensionless);

t is the time, T;

W=W(x,y,t) is the volume flux per unit area (positive sign for outflow and negative for inflow), L/T; and

are the Cartesian coordinates, L.

The equation used to describe the two-dimensional areal transport and dispersion of a given nonreactive dissolved chemical species in flowing ground water may be written as

$$\frac{\partial (Cb)}{\partial \dot{x}} = \frac{\partial}{\partial \dot{x}_{\dot{i}}} (b \mathcal{D}_{\dot{i}\dot{j}} \frac{\partial C}{\partial \dot{x}_{\dot{j}}}) - \frac{\partial}{\partial \dot{x}_{\dot{i}}} (b C V_{\dot{i}}) - \frac{C'W}{\varepsilon} \qquad \dot{i}, j = 1, 2$$
 (2)

where

C is the concentration of the dissolved chemical species, M/L³

 v_{ij} is the coefficient of hydrodynamic dispersion (a second-order tensor), L^2/T ;

b is the saturated thickness of the aquifer, L; and

C' is the concentration of the dissolved chemical in a source or sink fluid M/L^3

Because aquifers have variable properties and complex boundary conditions, exact analytical solutions to the partial differential equations of flow (eq 1) and solute transport (eq 2) cannot be obtained directly. Therefore, approximate numerical methods must be employed.

The numerical methods require that the area of interest be subdivided by a grid into a number of smaller subareas. The model described herein utilizes a rectangular, uniformly spaced, block-centered, finite-difference grid, in which nodes are defined at the centers of the rectangular cells.

The accuracy of the numerical solution to the solute-transport equation has been evaluated by comparison of the numerical solution with available analytical solutions. One such comparison is shown in Figure Al which depicts the concentration distribution for the problem of steady state radial flow through a homogeneous, isotropic porous medium in which a well continously injects a tracer with concentration C_0 . This represents a severe test of the model. Nevertheless, as demonstrated by Figure Al, there is good agreement between the analytical and numerical solutions at two different times.

TECHNIQUES OF WATER-RESOURCES INVESTIGATIONS

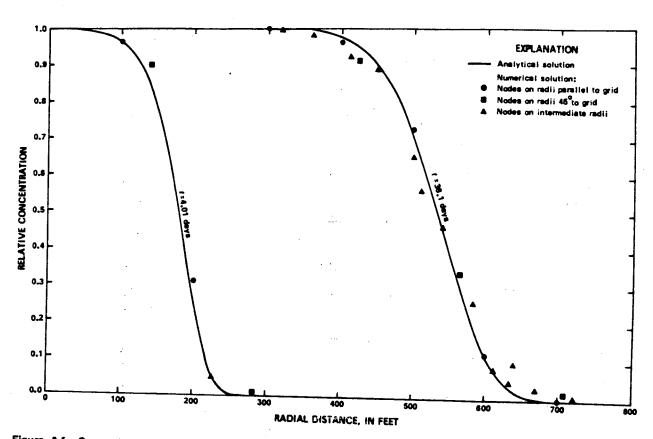


Figure A1 -Comparison between analytical and numerical solutions for dispersion in plane radial steady-state flow.

Figure 3 shows the geology of the site as interpreted from the following data;

- o 81 (approx.) test borings ranging in depth up to 400 feet.
- o 5300 feet of seismic refraction surveys.
- o 5600 feet of electromagnetic surveys
- o 3330 feet of downhole geophysical logging.
- o detailed geologic mapping.
- o an exhaustive literature review and personal communications with the U.S. Geological Survey and geologists familiar with the area.

Figure 3 is included to show the spatial relationship between geologic and hydrogeologic features and the locations of boreholes and other site data collection activities.

Residual materials encountered in the borings for the Phase II monitoring wells (MW-6, MW-7 and MW-8) consisted of silty clays and clayey silts of the Juana Diaz formation. These materials were indistinguishable from materials in exploratory borings penetrating the Juana Diaz formation drilled during the site characterization phase and during Phase I of the hydrogeologic assessment. Five of six samples tested from this formation contained greater than 70 percent fines (material passing a 0.074mm sieve opening). (See grain size curves in Appendix.) A significant percentage of clay minerals is indicated by the grain size distributions, which show greater than 25 percent clay-sized particles in four of six samples tested, and by the medium to high plasticity of the soils (liquid limits ranging from 38 to 93; plasticity indices ranging from 16 to 54).

Data available at this time indicate that the main westnorthwesterly trending faults act as hydrogeologic barrier
separating ground water in the Juana Diaz formation south of the
faults from the ground water in the Ponce formation north of
fault. Another fault, which occurs on the south side of the site
also acts as a hydraulic barrier which separates ground water in
the Juana Diaz formation from the Ponce. Additional water level,
water chemistry and geologic data obtained during and following
installation of the monitoring wells indicate the presence of
several fault blocks within the Juana Diaz formation. These
fault blocks, plus the steeply dipping beds and impermeable
unsaturated zone, are controlling the occurrence and movement of
ground water within the Juana Diaz formation and in the cell
area.

1.2.2.2 Potentiometric Levels

During drilling, the depth at which ground water is first encountered cannot always be determined exactly. It depends, to some extent, upon the drilling method being used. In air-rotary drilling the first encounter with water may be either abrupt or gradual. If the drill penetrates a hard confining layer and abruptly breaks into a saturated zone, the cuttings and soil cores from the hole will suddenly be saturated. In this case the location of the saturated zones can be determined with a high degree of certainty. In other cases there may be a capillary zone or a perched zone at some distance above the aquifer. In this situation the first encounter with the aquifer cannot be determined exactly because either the change in moisture content

is gradual or, in those cases where the moist cuttings cannot be removed from the hole by air lifting, it is necessary to use water as the drilling fluid. Once drilling water fills the hole, it is much more difficult to determine where ground water is encountered.

Water levels in monitoring wells drilled in the Juana Diaz formation were found at varying elevations irrespective of depth of the hole. These data along with geologic and water chemistry information indicate that known fault blocks within the Juana Diaz formation may not be hydraulically interconnected. This would imply that ground water occurs in tightly trapped pockets within the faulted Juana Diaz and that each of these pockets is independent of both a regional aquifer system and other fault blocks. The following is a discussion of water level variations encountered during drilling of the monitoring wells.

The potentiometric levels in some of the monitoring wells are above the level where water was first encountered in the boring, indicating confined or artesian conditions. This was clearly the case in three of the monitoring wells. As shown on Table 2, an increase in the ground water elevation of about 8.5 feet occurred in MW-3 after the first encounter with water was made at elevation 172.8 feet msl. Larger rises were noted in MW-7 and MW-8 where the increases in ground-water levels were about 37 and 139 feet, respectively.

Moist soil was encountered at an elevation of 153.7 in MW-1. The hole was then drilled about 30 feet deeper and bailed. After bailing, water reentered the hole and the assumption was made that a saturated zone had been encountered. Subsequently, the

water level in MW-l declined until eventually the hole became dry. MW-l was then replaced by MW-6. Moist soil was encountered in MW-6 at an elevation of approximately 72 feet msl. This hole was then extended to an elevation of 15 feet msl using water for drilling. Periodic bailing was conducted to check for the presence of ground water. A stable water level in MW-6 was determined to be about 46 feet msl. In drilling both MW-1 and MW-6, circulation of the drilling water was lost, indicating that some portions of the formation were absorbing this fluid.

In drilling MW-2, air was used until a saturated sample was recovered. The breakthrough into the saturated zone was abrupt, and the location of this zone was easily determined. The same was true for MW-3 and MW-7.

However, in MW-2 two different water-bearing zones in the main Car Juana Diaz formation were encountered during drilling of MW-2. ្នកនៅ។ While advancing the borehole using air-rotary methods, greenish esta eta gray clayey silt cuttings were noted at a depth of 23 feet. ---V/ When the drill rods and split spoon were retrieved after sampling 200 55 at a depth of 29 feet, the last several feet of rods and the Sact Sag split spoon were dripping wet. Drilling was suspended and water levels recorded in the open borehole. In thirty minutes the 2753 water rose 2.4 feet in the borehole indicating a minimum expected head of 26.6 feet below ground surface. The borehole was then advanced using wash-rotary methods to 60 feet. (The procedure used for the construction of the monitoring wells was to drill to a depth of 30 feet below the elevation of the water table.) 32 feet below ground surface, a strata change occurred from

direction. If a leak occurred directly into a fault, the contaminant would move away from the fault toward the monitoring wells.

La Cost (4) 20 Compatible

On the other hand, if the fault provided a continuous high W Lilis permeability path in the direction of the fault, the fault might . 33 *z*, 3, tend to act as a drain rather than a recharge zone, and the ජ ක්වෙම් ඉදි ground water levels near the fault would be lower than that in المعتد the surrounding rock. This is not the case at Cell Number 1 aw acity where the highest potentiometric values are those nearest the 32 35W fault that crosses Cell Number 1, i.e., at MW-2, MW-3 and MW-7.

odnit light Evidence is stronger that the faults are acting as barriers to ground water flow. The "fault as a barrier" was discussed in the SCR. Data obtained early in the study of the Ponce site and Tellio tel reported in the SCR showed the main fault crossing the site and ripophe kit separating the Ponce from the Juana Diaz to act as a barrier to **ារខែ**ទំហា**ខ**ាស់ជាទីស ស. ស. ស. ស. ស. ground water flow. This evidence has been strengthened by data subsequently obtained from MW-8. MW-8 has a water level of about 833W 0. og tidliga omer 150 feet MSL. It is only about 200 feet south of the fault. -26. 3df the north side of the fault, in the Ponce formation, water level I . Jim in C-15 was 15 feet MSL. If the fault were not acting as a barrier, the ground water in the Juana Diaz would flow into and ಾಂದ ಗರ್ಚಾಸ್ತ್ರಿಸ The Dorone. come into equilibrium with the ground water in the Ponce forma-ត[្] ជិតបេសមន្ត tion. The evidence shows this has not happened. that ೨೯ನಿಕ್ಕಾರ ತೃತ್ತಿ ಅ

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ar**e**for sure on the object

MW-3, MW-4 and MW-1 due to the breakdown of the steam cleaning rig. Prior to drilling MW-2 and completion of drilling at MW-5 (48 to 64 feet) the drill rig and equipment were mobilized to a nearby fire station. At the fire station tools and equipment were placed on sawhorses and the rig and equipment thoroughly cleaned using potable water under very high pressure flowing through a fire hose connected to a fire hydrant.

Permeability Testing

Field permeability tests were performed in six of the eight wells installed at the test boring locations. These permeability tests or "slug" tests were performed by raising or lowering the water level in the well with a weighted float (slug in) or bailer (slug out) and subsequently measuring the gradual water level decline or rise with an electric tape. In the case of "slug in", once the water level reached static conditions the float was removed from the well (slug out) and the recovering water level was measured.

Water level information was plotted on semilog graph paper on the linear axis against time in seconds on the log axis (Lohman, 1979). A family of matching curves were overlain on the data plot so that the plotted data points best fit one of the type curves. A value of time (t) in seconds on the data plot was picked where the data coordinate was found to overlie the value of $Tt/rc^2 = 1.0$ on the type curve. Once the value of t (seconds) was obtained it was used in the equation: